### PLANNING PROPOSAL TO PERMIT MIXED USE REDEVELOPMENT

152 – 206 ROCKY POINT ROAD, KOGARAH

Assessment of Traffic and Parking Implications

> January 2015 (Rev E)

Reference 13091

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## TABLE OF CONTENTS

1.	INTF	RODUCTION	1
2.	PRO	POSED REZONING SCHEME	3
	2.1 2.2	Site, Context And Existing Use Planning Proposal	3 4
3.	ROA	D NETWORK AND TRAFFIC CONDITIONS	5
	3.1 3.2 3.3 3.4 3.5	Road Network Traffic Controls Traffic Conditions Transport Services Future Circumstances	6 6 7
4.	ACC	ESS AND CIRCULATION	8
5.	TRA	FFIC 1	10
6.	PAR	KING AND SERVICING1	15
7.	TRA	NSPORT SERVICES, BICYCLES AND PEDESTRIANS 1	6
8.	CON	ICLUSION 1	17

APPENDIX A	INTERSECTION PLAN
APPENDIX B	SURVEY RESULTS
APPENDIX C	SIDRA RESULTS
APPENDIX D	EXTRACTS FROM RTA H.D. RESIDENTIAL STUDY
<b>APPENDIX E</b>	<b>RMS W</b> ARRANTS

## LIST OF ILLUSTRATIONS

- FIGURE 1ECONTROLFIGURE 2SITEFIGURE 3ROAD NETWORKFIGURE 4TRAFFIC CONTROLSFIGURE 5EXISTING PEAK TRAFFIC MOVEMENTS

## 1. INTRODUCTION

This report has been prepared to accompany a Planning Proposal to Rockdale City Council for a rezoning to permit residential apartment, office and showroom/bulky goods development on a large consolidated site at 152 – 206 Rocky Point Road, Kogarah (Figure 1).

Many established suburban areas throughout the Sydney Metropolitan Area, particularly those with convenient access to public transport and employment opportunities, are experiencing ongoing redevelopment as part of the urban consolidation process. Older style industrial sites and redundant uses are generally being consolidated and replaced with residential apartments often associated with contemporary mixed use elements.

The site for the proposed rezoning at Kogarah has convenient access to the arterial road system and public transport links to the City and the expanding airport/port precincts and presents an ideal opportunity for redevelopment. The envisaged development consequential to the proposed rezoning based on the indicative Masterplan prepared by Lippman Partnership would comprise up to:

- 453 apartments with basement carparking
- 20,093m<sup>2</sup> non residential floorspace
  - 14,286m<sup>2</sup> office
  - 4,393m<sup>2</sup> showroom/bulky goods
  - 1,414m<sup>2</sup> warehouse
- a new access road system



The purpose of this report is to:

- describe the site, its context and the potential development outcome as a result of the Planning Proposal
- \* describe the road network and traffic conditions in the area
- assess the suitability of the proposed vehicle access and circulation arrangements
- assess the potential traffic implications and identify and necessary road/traffic managements upgrade works
- \* assess the potential, parking, internal circulation and servicing implications

## 2. PROPOSED REZONING SCHEME

#### 2.1 SITE, CONTEXT AND EXISTING USE

The development site (Figure 2) is a large irregular shaped consolidation of numerous lots occupying a total area of some 33,488m<sup>2</sup>. The site, which extends between Rocky Point Road and Scarborough Park, has an extensive frontage to the eastern side of Rocky Point Road with a slight fall in levels towards the eastern boundary.

The existing buildings on the site comprise:

- **\*** 17,040m<sup>2</sup> industrial
- 1,600m<sup>2</sup> warehouse
- **\*** 1,650m<sup>2</sup> showroom/bulky goods
- **\*** 1,750m<sup>2</sup> office
- ★ 1 residential dwelling

The surrounding uses include:

- \* industrial buildings adjoining to the north
- \* Calvary Hospital just to the north
- \* Scarborough Park open space and recreation/sport areas to the east
- mixed uses extending along Rocky Point Road including a number of new multi-level building

The Kogarah Town Centre and railway station are located some 2 kms to the north while the small Ramsgate Centre is some 500 metres to the south.



#### 2.2 PLANNING PROPOSAL

The Planning Proposal for rezoning seeks an FSR of 2:1 which would provide for more than 60,000m<sup>2</sup> GFA to be achieved. The development concept identifies showroom/bulky goods and commercial office development along the western part (Rocky Point Road frontage) and residential apartment buildings on the central and eastern part.

The existing structures on the site, apart from the Harvey Norman offices and showroom/warehouse of 3,750m<sup>2</sup> and 63 basement parking spaces, would be demolished. Based on the indicative Masterplan the western part of the site would be developed for non residential uses which is envisaged to comprise approximately:

4,393m<sup>2</sup> showroom/bulky goods 14,286m<sup>2</sup> commercial office 1,414m<sup>2</sup> warehouse

The central and eastern parts of the site would be developed for residential apartment buildings extending along a new access road system. A total of 453 apartments are envisaged with a make up of 1 Bed (128), 2 Bed (250) and 3 Bed (75). All car parking would be contained in single basement levels apart from some indented on-street spaces with a total of some 1,003 spaces.

The principal access road will connect with Rocky Point Road opposite Weeney Street and a shared pedestrian/bicycle path would be provided along the northern side of the new road connecting as an extension to the proposed shared path system through Scarborough Park.

Details of the envisaged development are provided on the diagrams prepared by Lippmann Partnership which accompany the Planning Proposal and are reproduced in part overleaf.

RFDC REQUIREMENT COUNCIL DCP 10-30% 50-75% 10-30% 20 093 m2 1.8:1 **B6 ENTERPRISE CORRIDOR ZONE** 11 163 m2 33 488 m2 COUNCIL PROPOSED HEIGHT (m) 2874 1 640 (37% OF COMM. (25% OF COMM. OPEN SPACE) OPEN SPACE) EXISTING HARVEY NORMAN NEW COMMERCIAL 22 325 m2 44 650 m2 2.00:1 BLOCK (G) AND NUMBER OF LEVELS (3) 90 DARRELL LEA 30 HARVEY NORMAN 120 5 581 <sup>25%</sup> LEGEND FOR RESIDENTIAL BLOCK NOTATION PROPOSED HEIGHT (m) **90 DARRELL LEA R4 RESIDENTIAL ZONE** 3 280 16 813 128 (29.2%) 250 (57.1%) 60 (13.7%) S: 15 453 PROVIDED TOTAL SITE AREA: EMPLOYMENT TABLE 7693 <sup>35%</sup> SITE AREA: FLOOR SPACE: EXISTING PROPOSED TOTAL TOTAL DWELLINGS: 25 378 403 UNIT MIX 1 BED UNITS: 12 2 BED UNITS: 23 3 BED UNITS: 23 1 TOWNHOUSES: SITE AREA: Floor Space: FSR COMMUNAL OPEN SPACE 2. CURRENTLY PROPOSED TOTAL = TOTAL = 8.5 8.5 DEEP SOIL ZONE 1. PRE 2010 ЗL Ċ FSR: ć 10050 4000 LIDING POTENTIAL SITE FOR COMMUNITY BUILDING 10008 1 0017 JOIE 0066 0007! 9004 34000 09691 00991 PRODUCTION LANE **PRODUCTION AVE** 32.0m 10L 13.4m 4L 3 ന F , N 25.8 81  $\nearrow$ 13.4m 4L 24000  $\overleftarrow{}$ ENTRY **R4 RESIDENTIAL ZONE**  $\times$ 32.0m 10L д 7L TE X **N** 2 32300 24000 18800 13.4m 4L A25.8m 8L 32.0 10L 50600 Э ∡  $\mathbb{X}$ 13000 16.5m 5L ΣĘ +15600 -2400 M 8.5m 2.5L 28.9m 9L  $\times$ CARPAR! ENTRY / EXIT 1 32.0m 10L 00051 000 BUL CARPARK ENTRY / EXIT  $\boxtimes$ ENTRY EXIT COMM C 18m 4L  $\square$ 

ATTACHMENT A.3 MASTERPLAN FOR OPTION 2(a)

152-206 ROCKYPOINT ROAD, ROCKDALE, PLANNING PROPOSAL

Lippmann Pathership Masterplanning Lippmann Pathership Architecure 510 Cown Street Surry Hills NSW 2010 Surry Hills NSW 2010 Surry Hills Area

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## 3. ROAD NETWORK AND TRAFFIC CONDITIONS

#### 3.1 ROAD NETWORK

The road network serving the site (Figure 3) comprises:

- Princes Highway a State Highway and principal north-south arterial route to/from the City
- General Holmes Drive/Grand Parade a State Road and arterial route linking between Mascot and Sandringham along the western side of Botany Bay
- President Avenue a State Road and sub-arterial route linking between
  Princes Highway and Grand Parade
- *Rocky Point Road* a State Road and sub-arterial route linking between Princes Highway and Captain Cook Bridge
- *Ramsgate Road and Jubilee Avenue* collector roads through the Kogarah/
  Ramsgate areas connecting to the arterial routes
- \* Production Avenue and Phillips Road local access roads
- \* *Production Lane* a narrow service lane

Rocky Point Road in the vicinity of the site is some 13m wide with 2 traffic lanes in each direction with a relatively straight and level carriageway. Production Avenue and Phillips Road have relatively narrow roadways some 9m wide while Production Lane is a narrow informal service road.





#### 3.2 TRAFFIC CONTROLS

The existing traffic controls which have been applied to the road system in the vicinity of the site are shown on Figure 4. The principal features of these controls are:

- the traffic control signals along Rocky Point Road including the Jubilee Avenue/Phillips Road intersection (Appendix A plan) and the pedestrian signals to the north of Sunbeam Avenue
- the one-way traffic flows along Phillips Road (westbound) and Production Avenue (eastbound)
- the CLEARWAY restrictions along Rocky Point Road (westside AM peak and eastside PM Peak)
- the BUS ZONE and sections of NO STOPPING/NO PARKING restrictions along Rocky Point Road
- the roundabouts on Jubilee Avenue at the Burgess Street intersection and on Weeney Street at the Carrol Street and Burgess Street intersections
- the pedestrian refuge in Rocky Point Road at the southern part of the site frontage

#### **3.3 TRAFFIC CONDITIONS**

An indication of the existing traffic conditions in the vicinity of the site is provided by data published by RMS<sup>1</sup> and surveys undertaken as part of this study. The RMS data is expressed in terms of Annual Average Daily Traffic (AADT) as follows:

Location	AADT
Rocky Point Road south of Princes Highway	24,039
Ramsgate Road east of Rocky Point Road	13,064



#### TRANSPORT AND TRAFFIC PLANNING ASSOCIATES

Traffic surveys have been undertaken during the morning and afternoon peak periods at the intersections of Jubilee Avenue/Phillips Road/Production Avenue and Weeney Street with Rocky Point Road. The results of those surveys are provided on Figure 5 indicating fairly strong peak directional flows (ie northbound in the morning and southbound in the afternoon) on Rocky Point Road.

The operational performance of the Rocky Point Road, Jubilee Avenue and Phillips Road intersection has been assessed using SIDRA. The results of that assessment indicating a satisfactory performance are provided in the following while the criteria for interpreting SIDRA results is reproduced overleaf.

	AM	PM
Level of Service	А	А
Deg. of Sat.	0.562	0.758
Av. Vehicle Delay	10.0	10.8

#### 3.4 TRANSPORT SERVICES

Public transport plays an important role in meeting the travel needs of residents, employees and visitors of the area. The development site is located some 2.0 kms from Kogarah Railway Station on the Illawarra Line and there are a number of bus services operating in the area (STA Routes 476-77 along Rocky Point Road) which provide linkages to the railway station and other centres and the CBD

#### 3.5 FUTURE CIRCUMSTANCES

The only potential significant road network development in the area is the construction of an arterial route along the "F6 Corridor" which runs through Scarborough Park. Although this project is raised in Government from time to time there is no current plan or time frame for this work to be undertaken.

## **Criteria for Interpreting Results of SIDRA Analysis**

#### 1. Level of Service (LOS)

LOS	Traffic Signals and Roundabouts	Give Way and Stop Signs
'A'	Good	Good
'B'	Good with acceptable delays and spare capacity	Acceptable delays and spare capacity
'C'	Satisfactory	Satisfactory but accident study required
'D'	Operating near capacity	Near capacity and Accident Study required
'E'	At capacity; at signals incidents will cause excessive delays. Roundabouts require other control mode	At capacity and requires other control mode
'F'	Unsatisfactory and requires additional capacity	Unsatisfactory and requires other control mode

## 2. Average Vehicle Delay (AVD)

The AVD provides a measure of the operational performance of an intersection as indicated on the table below, which relates AVD to LOS. The AVD's listed in the table should be taken as a guide only as longer delays could be tolerated in some locations (ie inner city conditions) and on some roads (ie minor side street intersecting with a major arterial route).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabouts	Give Way and Stop Signs
А	Less than 14	Good operation	Good operation
В	15 to 28	Good with acceptable delays and spare capacity	Acceptable delays and spare capacity
С	29 to 42	Satisfactory	Satisfactory but accident study required
D	43 to 56	Operating near capacity	Near capacity and accident study required
E	57 to 70	At capacity; at signals incidents will cause excessive delays. Roundabouts require other control mode	At capacity and requires other control mode

## 3. Degree of Saturation (DS)

The DS is another measure of the operational performance of individual intersections.

For intersections controlled by **traffic signals**<sup>1</sup> both queue length and delay increase rapidly as DS approaches 1, and it is usual to attempt to keep DS to less than 0.9. Values of DS in the order of 0.7 generally represent satisfactory intersection operation. When DS exceeds 0.9 queues can be anticipated.

For intersections controlled by a **roundabout or GIVE WAY or STOP signs**, satisfactory intersection operation is indicated by a DS of 0.8 or less.

<sup>&</sup>lt;sup>1</sup> the values of DS for intersections under traffic signal control are only valid for cycle length of 120 secs

## 4. ACCESS AND CIRCULATION

The large site is currently reliant on accesses located on the Rocky Point Road frontage apart from some connection to the Production Lane which connects to Production Avenue and Phillips Road. At the same time Phillips Road and Production Avenue are very narrow roadways with consequential one way traffic flow restrictions.

It is entirely evident that development of the nature envisaged (subject to approval of the Planning Proposal) will require an appropriate new access road system with suitable traffic management on the connection to Rocky Point Road.

The proposed access road system comprises:

- a central 2 way access roadway connecting between Rocky Point Road and Production Lane. This 7m wide roadway will have indented parking bays with widening at the Rocky Point Road connection (ie 2 approach lanes)
- **\*** formalisation of the Production Lane
- minor "secondary" roadway connecting between the Production Lane and the main access road 7m wide

The vehicle access connection on Rocky Point Road for the existing Harvey Norman complex will be retained.

The new central access roadway will connect with Rocky Point Road directly opposite Weeney Street and it is proposed that this 4 way junction will be controlled by traffic signals. Apart from providing for the access movements generated by the envisaged development these traffic signals will provide for:

- pedestrian crossing movements across Rocky Point Road, particularly to/from the bus stops (ie the distance between the existing Ramgate Road and Jubilee Ave traffic signals on Rocky Point Road is extensive and pedestrians could not be expected to divert to these crossing points) which are currently assisted with a pedestrian refuge island
- pedestrian and cyclist movements which will travel along the proposed shared pathway along the new central access road with connection to the shared pathway system through Scarborough Park
- \* vehicles which access to/from Weeney Street

The proposal to provide traffic signals at the new access road intersection has been discussed with RMS and no objection was raised as long as the need satisfies the RMS "warrants".

The issue of satisfying the RMS warrants for traffic signals is addressed in Section 5 of this report.

## 5. TRAFFIC

An indication of the potential traffic generation of the envisaged development elements is provided by RMS development guidelines (as updated) which specify the following:

Showroom/bulky good	-	2.7 vtph per 100m <sup>2</sup> GFA *	
Office	-	AM1.6 vtph per $100m^2$ PM1.2 vtph per $100m^2$	
Warehouse	-	0.5 vtph per 100m <sup>2</sup>	
Residential Apartment	-	0.29 vtph per dwelling **	

\* While TDT 04-2013 specifies that the morning site peak does not generally coincide with the network peak there are staff arrivals, servicing and there will potentially be some elements which will generate at this time (eg café, takeaway food)

\*\* While TDT 04-2013 specifies a lower rate this is for buildings 6 levels and more with high transport accessibility (eg railway station). Details of surveys of comparable location/nature are provided in the extracts from the earlier RTA study reproduced in Appendix D.

Application of this criterion to the envisaged development would indicate the following:

14,286m <sup>2</sup> office		453 apartments	
AM 1.6 vtph per 100m <sup>2</sup>	229 vtph	at 0.29 vtph per apartment	131 vtph
PM 1.2 vtph per 100m <sup>2</sup>	171 vtph		
4,393m <sup>2</sup> showroom/bulky g	goods	1,414m <sup>2</sup> warehouse	
<b>4,393m<sup>2</sup> showroom/bulky g</b> AM 1.7 vtph per 100m <sup>2</sup>	<b>joods</b> 75 vtph	<b>1,414m<sup>2</sup> warehouse</b> AM	7 vtph
	75 vtph		7 vtph 7 vtph



#### TRANSPORT AND TRAFFIC PLANNING ASSOCIATES

Additional to this traffic at the intersection will be:

- \* traffic generated by uses in Production Avenue which is egressing to the south
- \* the existing Weeney Street movements
- traffic ingressing from the south to the uses in Phillips Road which choose to turn right into the new Access road rather than travel via Production Avenue (in fact RMS may choose to prohibit the right turn into Production Avenue if a signal controlled turn is available at the new access road)
- traffic in the precinct around Weeney Street which will choose to access through the new signals

The projected distribution of the movements generated by the envisaged development is as follows:

	AM		PM	
	IN	OUT	IN	OUT
Office	210	19	19	152
Showroom/BG's	68	7	54	71
Warehouse	5	2	2	5
Residential	26	105	105	26
Total:	309	133	180	254

The projected directional distribution is as follows:

North	65%
South	30%
West	5%

The resultant volumes at the Rocky Point Road access intersection including the additional movements attracted by the traffic signal provision are shown on Figure 6 and summarised in the following:

		AM	РМ
Rocky Point Road	NB	1,370	762
	RT	122	61
	LT	59	24
	SB	574	1,299
	LT	96	121
New Road	WB	21	23
	RT	205	170
	LT	105	108
Weeney Street	EB	23	24
	RT	52	57
	LT	28	24
Rocky Point Road		2,099	2,206
Total conflicting flow with Rocky Point Road flow		556	455

The person trip generation rates per apartment as established in the recent RMS survey was 0.80 ptph in the morning and 0.60 ptph in the afternoon with a mode share to car of 40 to 50%. Thus the proposed 453 apartments could generate some 175 to 200 pedestrian movements in the morning peak and some 130 to 158 pedestrian movements in the afternoon.

Similar analysis of the data for showroom/bulky goods uses and office uses with office rates of some 2.16 ptph per 100m<sup>2</sup> in the morning peak and 1.73 in the afternoon with a non-car mode of some 30%. The result and pedestrian generation is some 200 to 230 ptph.

Thus the total pedestrian generation rate for the envisaged development is some 300 to 400 pph in the peak periods and in the morning peak many will cross Rocky Point Road to catch the City/Kogarah/Rockdale bound buses. It is also noted that the recent large nearby mixed use developments have also generated heightened pedestrian crossing activity in the vicinity.

#### TRANSPORT AND TRAFFIC PLANNING ASSOCIATES

The operational performance of the proposed access intersection and the Rocky Point Road/Jubilee Ave/Phillips Road intersection with the projected additional traffic volumes has been assessed using SIDRA. The results of that assessment, including signalised and non-signalised options for the access intersection, are summarised in the following:

Access Intersection	Signalised	Non-signalised
AM Peak		
LOS	А	F
DS	0.763	4.596
AVD	11.8	449.4
PM Peak		
LOS	А	F
DS	0.801	4.477
AVD	10.2	385.8
Julibee/Phillips Intersection		
AM Peak		
LOS	А	
DS	0.651	
AVD	9.8	
PM Peak		
LOS	В	
DS	0.888	
AVD	14.1	

The results indicated that:

- the access intersection will be entirely unsatisfactorily without the provision of traffic signals
- \* the signalised intersections will operate quite satisfactorily

In relation to the issue of RMS warrants for traffic signals these are outlined in the extract reproduced in Appendix E. It is noted that these long established warrants are somewhat simplistic and do not have regard for:

- right turning volumes
- combined minor road and pedestrian crossing volumes

However, reference to the vehicle arrival/departure and person trip movements for the various landuses indicate that the peak volumes occur for at least 2 hours in both the morning and afternoon periods (ie the warrant for 4 hours per day is met). It is also clear that the vehicle volume warrant is met and this would be supported by the pedestrian crossing need.

## 6. PARKING AND SERVICING

#### PARKING

Parking would be provided commensurate with Council's DCP requirements with some 498 spaces for residents, some 91 for resident visitors and appropriate bicycle and motor bike spaces. Some 414 spaces would be provided for the showroom/bulky goods and office elements with appropriate bicycle and motor bike provisions.

Some indented bays would be provided along the new access road system which would contribute to the visitor parking requirements.

#### SERVICING

Refuse would be removed from the street by Council's collection service and the access road system is designed facilitate the movements of these vehicles. Service personnel will be able to use the visitor spaces while other service/delivery vehicles will:

- use the dock areas provided in the commercial buildings
- use the on-street bays

## 7. TRANSPORT SERVICES, BICYCLES AND PEDESTRIANS

#### **TRANSPORT SERVICES**

The existing bus stops on Rocky Point Road at the site frontage will provide for the transport needs of residents, workers and visitors generated by the proposed development.

The bus services connect to Kogarah and Rockdale Railway Stations where linkages are available to the Metropolitan transport network.

The bus services which operate along The Grand Parade are within easy walking distance and provide connection to/from the City.

#### **BICYCLES AND PEDESTRIANS**

The new access road network will incorporate appropriate provision for bicycles and pedestrians. In particular it is proposed to provide a shared pathway along the principal access road which will link to the proposed shared path to be provided through Scarborough Park. The movements of pedestrians and cyclists across Rocky Point Road will be facilitated by the proposed traffic signals on Rocky Point Road.

## 8. CONCLUSION

The proposed rezoning of the consolidated Industrial zoned site at Kogarah to enable a mix of residential and non residential development will represent an appropriate outcome consistent with the surrounding area. Adequate parking will also be provided to accommodate the proposed development while the minor servicing activity will be accommodated on-site as is appropriate to the nature of these activities.

There will not be any adverse or unsatisfactory traffic or parking implications resulting from the redevelopment scheme.

# APPENDIX A

## INTERSECTION PLAN



# APPENDIX B

## **SURVEY RESULTS**



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0645 - 0700

		TOTAL	299	342	449	504	493	530	529	532	459	449	4586	
	Point	ы	212	236	308	348	350	350	352	345	276	275	3052	
	Rocky Point	IL	1	1	10	7	10	15	18	16	6	5	92	
VEOL	Weeny St	R	5	8	5	5	5	5	9	9	7	7	59	
	Weer	Ē	6	7	10	6	9	7	5	10	6	10	82	
	Rocky Point	н	72	89	111	133	122	152	146	154	155	150	1284	
	Rocky	R	0	1	5	2	0	1	2	1	3	2	17	
Ledestilatis		Time Per	0630 - 0645	0645 - 0700	0700 - 0715	0715 - 0730	0730 - 0745	0745 - 0800	0800 - 0815	0815 - 0830	0830 - 0845	0845 - 0900	Period End	

	ION	NORTH	WEST	ST	SOUTH	ЛΤΗ	
	Rocky	Rocky Point	меел	Neeny St	Rocky Point	Point	
Peak Per	R	г	L	R	Г	Г	TOTAL
0630 - 0730	8	405	35	23	19	1104	1594
0645 - 0745	8	455	32	23	28	1242	1788
0200 - 0800	8	518	32	20	42	1356	1976
0715 - 0815	5	553	27	21	50	1400	2056
0730 - 0830	4	574	28	22	59	1397	2084
0745 - 0845	7	209	31	24	58	1323	2050
0800 - 0900	8	605	34	26	48	1248	1969



			TOTAL	516	498	461	454	528	550	531	493	567	503	5101
t	SOUTH	Rocky Point	Ы	199	185	181	154	196	178	189	173	227	185	1867
T.T.P.A. 4750 KOGARAH Weeny St Monday 5th August 2013	IOS	Rocky	Ē	3	6	9	2	7	5	8	4	7	10	61
T.T.P.A. 4750 KOGARAH Weeny Monday 5th August 2013	WEST	Weeny St	R	5	9	9	8	8	5	9	9	10	5	65
.A. KOGA ay 5th	ME	Meel	F	16	9	8	7	6	8	9	4	9	5	75
: T.T.P.A. : 4750 KC : Monday	NORTH	Rocky Point	Ы	288	289	257	279	302	348	320	302	311	293	2989
ame te	ION	Rocky	R	5	3	3	4	9	9	2	4	9	5	44
Client Job No/Name Day/Date	Pedestrians		Time Per	1530 - 1545	1545 - 1600	1600 - 1615	1615 - 1630	1630 - 1645	1645 - 1700	1700 - 1715	1715 - 1730	1730 - 1745	1745 - 1800	Period End

ТН	Point	<u>τ</u> τοται	719 <b>1929</b>	716 <b>1941</b>	709 <b>1993</b>	717 2063	736 <b>2102</b>	767 2141	774 2094
SOUTH	Rocky Point	Ē	20	24	20	22	24	24	29
WEST	Weeny St	<b>R</b>	25	28	27	27	25	27	27
ME	юәд	Ē	37	30	32	30	27	24	21
NORTH	Rocky Point	Ţ	1113	1127	1186	1249	1272	1281	1226
ION	Rocky	R	15	16	19	18	18	18	17
		Peak Per	1530 - 1630	1545 - 1645	1600 - 1700	1615 - 1715	1630 - 1730	1645 - 1745	1700 - 1800



# APPENDIX C

## SIDRA RESULTS

		Demand		Deg	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Tum	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South: E	Dealey Dt	veh/h	%	v/a	SEQ.		veh	m		per veh	km/l
South. F	Rocky Pt			0.500	45.0	LOCD		00.0	0.20	0.05	42.
1	-	117	2.0	0.562	15.6	LOS B	11.3	80.3	0.39	0.95	43.
2	t	1175	2.0	0.562	7.4	LOS A	11.4	81.0	0.39	0.35	48.3
Approac	ch	1292	2.0	0.562	8.1	LOS A	11.4	81.0	0.39	0.41	47.
East: Ph	hillips Rd										
4	L	20	2.0	0.277	60.0	LOS E	1.5	11.0	0.99	0.72	23.
5	т	11	2.0	0.277	51.7	LOS D	1.5	11.0	0.99	0.71	23.
6	R	16	2.0	0.146	59.4	LOS E	0.8	5.6	0.98	0.69	22.
Approad	ch	47	2.0	0.277	57.8	LOS E	1.5	11.0	0.98	0.71	22.
North: F	Rocky Pt F	Rd									
8	т	541	2.0	0.412	2.8	LOS A	1.9	13.4	0.17	0.15	54.
9	R	45	2.0	0.412	10.0	LOS A	0.5	3.3	0.09	0.86	47.
Approad	ch	586	2.0	0.412	3.4	LOS A	1.9	13.4	0.17	0.21	54.
West: J	ubilee Av	Э									
10	L	138	2.0	0.257	8,8	LOS A	0.5	3.3	0.19	0.69	48.
12	R	105	2.0	0.358	49.7	LOS D	4.7	33.4	0.93	0.78	25.
Approad	ch	243	2.0	0.358	26.5	LOS B	4.7	33.4	0.51	0.73	34.
All Vehi	cles	2168	2.0	0.562	10.0	LOSA	11.4	81.0	0.36	0.40	46.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

	ent Performance -	Demand	Average	Level of	Average Back	ofQueue	Prop.	Effective
Mov ID	Description	Flow ped/h	Delay	Service		Distance	Queued	Stop Rate
P5	Across N approach	53	44.2	LOS E	0.1	0.1	0.94	0.94
All Pede	estrians	53	44.2	LOS E			0.94	0.94

Level of Service (LOS) Method; SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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woven	ient Per	formance - V	enicies						-		
Mov ID	Tum	Demand Flow veh/h	HV %	Deg Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	or Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: F	Rocky Pt								11.54		
1	L	111	2.0	0.180	13.5	LOSA	1.3	9.0	0.23	0.72	43.9
2	т	885	2.0	0.758	9.0	LOS A	21.5	153.1	0.55	0.50	46.2
Approa	ch	996	2.0	0.758	9.5	LOS A	21.5	153.1	0.51	0.53	45.9
East: Pl	hillips Rd										
4	L	37	2.0	0.666	62.6	LOS E	3.9	27.9	1.00	0.81	22.5
5	т	38	2.0	0.666	54.4	LOS D	3.9	27.9	1.00	0.81	22.6
6	R	30	2.0	0.273	60.2	LOS E	1.5	10.7	0.99	0.72	22.5
Approa	ch	105	2.0	0.666	58.9	LOS E	3.9	27.9	1.00	0.79	22.5
North: F	Rocky Pt I	Rd									
8	т	893	2.0	0.619	2.7	LOS A	4.2	29.8	0.18	0.16	54.8
9	R	72	2.0	0.619	15.2	LOS B	4.2	29.7	0.36	0.90	43.3
Approa	ch	965	2.0	0.619	3.7	LOS A	4.2	29.8	0.19	0.21	53.7
West: J	ubilee Av	e									
10	L	93	2.0	0.191	8.8	LOS A	0.4	2.6	0.14	0.68	48.3
12	R	85	2.0	0.290	49.2	LOS D	3.7	26.7	0.92	0.77	25.4
Approa	ch	178	2.0	0.290	28.1	LOS B	3.7	26.7	0.51	0,73	33.8
All Vehi	cles	2244	2.0	0.758	10.8	LOSA	21.5	153.1	0.40	0.42	45.3

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Mover	ment Performance -	Pedestrian	5					
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	King and Setting and Section 1.	of Queue Distance m	Prop. Queued	Effective Stop Rate
P5	Across N approach	53	44.2	LOS E	0.1	0.1	0.94	0.94
All Ped	lestrians	53	44.2	LOS E			0.94	0.94

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements,

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at a local distance	111-10-10	formance - V Demand	ennerio -	Dea	Augrapa	Level of	DEW Book	of Outours	Prop	Effective	Augrage
Mov ID	Turn	Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Service	95% Back   Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: F	Rocky Pt					-					
1	L	117	2.0	0.651	16.3	LOS B	15.0	106.9	0.45	0.95	42.8
2	т	1380	2.0	0.651	8.0	LOS A	15.1	107.8	0.45	0.41	47.3
Approad	ch	1497	2.0	0.651	8.7	LOS A	15.1	107.8	0.45	0.45	46.9
East: Pl	hillips Rd										
4	L	5	2.0	0.141	59.0	LOS E	0.8	5.6	0.98	0.70	23.5
5	т	11	2.0	0.141	50.8	LOS D	0.8	5.6	0.98	0.68	23.7
6	R	16	2.0	0.146	59.4	LOS E	0.8	5.6	0.98	0.69	22.7
Approad	ch	32	2.0	0.146	56.4	LOS D	0.8	5.6	0.98	0.69	23.2
North: F	Rocky Pt I	Rd									
8	т	637	2.0	0.503	3.8	LOS A	2.7	19.1	0.25	0.22	53.2
9	R	45	2.0	0.503	10.7	LOS A	0.6	4.3	0.12	0.85	46.7
Approad	ch	682	2.0	0.503	4.2	LOS A	2.7	19.1	0.24	0.27	52.7
West: J	ubilee Av	0									
10	L	138	2.0	0.257	8.8	LOS A	0.5	3.3	0.19	0.69	48.1
12	R	105	2.0	0.358	49.7	LOS D	4.7	33.4	0.93	0.78	25.3
Approa	ch	243	2.0	0.358	26.5	LOS B	4.7	33.4	0.51	0.73	34.6
All Vehi	cles	2454	2.0	0.651	9.8	LOSA	15.1	107.8	0.41	0.43	46.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	Sec		ped	m		per ped
P5	Across N approach	53	44.2	LOS E	0.1	0.1	0.94	0.94
All Ped	estrians	53	44.2	LOS E			0.94	0.94

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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		Demand		Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Tum	Flow veh/h	HV %	Satn v/c	Delay sec	Service	Vehicles veh	Distance m	Queued	Stop Rate per veh	Speed km/h
South: I	Rocky Pt	Rd									
1	L	111	2.0	0.211	13.5	LOS A	1.5	10.7	0.24	0.75	43.9
2	т	1055	2.0	0.888	16.9	LOS B	39.9	284.1	0.74	0.75	39.1
Approa	ch	1166	2.0	0.888	16.6	LOS B	39.9	284.1	0.69	0.75	39.5
East: P	hillips Rd										
4	L	7	2.0	0.393	60.5	LOS E	2.3	16.1	1.00	0.74	23.4
5	т	38	2.0	0.393	52.2	LOS D	2.3	16.1	1.00	0.73	23.5
6	R	30	2.0	0.273	60.2	LOS E	1.5	10.7	0.99	0.72	22.5
Approach		75	2.0	0.393	56.2	LOS D	2.3	16.1	0.99	0.73	23.1
North: F	Rocky Pt I	Rd									
8	т	1014	2.0	0.796	4.3	LOS A	9.1	65.0	0.28	0.26	52.1
9	R	72	2.0	0.796	34.8	LOS C	7.2	51,3	0.86	0.89	31.5
Approach		1086	2.0	0.796	6.3	LOS A	9.1	65.0	0.32	0.30	50.0
West: J	ubilee Av	e									
10	L	93	2.0	0.191	8.8	LOS A	0.4	2.6	0.14	0.68	48.3
12	R	85	2.0	0.290	49.2	LOS D	3.7	26.7	0.92	0.77	25.4
Approach		178	2.0	0.290	28.1	LOS B	3.7	26.7	0.51	0.73	33.8
All Vehicles		2505	2.0	0.888	14.1	LOSA	39.9	284.1	0.53	0.55	41.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians									
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance M	Prop. Queued	Effective Stop Rate per ped	
P5	Across N approach	53	44.2	LOS E	0.1	0.1	0.94	0.94	
All Ped	lestrians	53	44.2	LOS E			0.94	0.94	

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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- 14 SIDRA INTERSECTION
## **MOVEMENT SUMMARY**

Giveway / Yield (Two-Way)

- and a second		Demand		Deg	Average	Level of	95% Back	of Queue	Prop	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Couthul	Rocky Pt I	veh/h	%	v/c	SEC	Contraction of the local distance	ven	m		per veh	km/t
South. I	ROCKY PI I			0.450	0.0	LOS A		0.0	0.00	1.05	49.0
	T	59	2.0	0.459	8.3		0.0	1.00078	25.001.0	0.00	52.8
2	11.5	1370	2.0	0.459	3.2	LOSA	7.6	54.4	0.38	10700	
3	R	122	2.0	0.459	16.3	LOS B	7.6	54.4	0.95	1.14	44,4
Approa	ch	1551	2.0	0.459	4.4	NA	7.6	54.4	0.41	0.13	51.9
East: A	ccess										
4	L	105	2.0	4.596	3309.1	LOS F	173.4	1234.8	1.00	6.24	0.7
5	т	21	2.0	4.596	3307.8	LOS F	173.4	1234.8	1.00	4.47	0.6
6	R	205	2.0	4.596	3309.2	LOS F	173.4	1234.8	1.00	4.39	0.6
Approa	ch	331	2.0	4.596	3309.1	LOS F	173.4	1234.8	1.00	4.98	0.6
North: F	Rocky Pt F	۶d									
7	L	96	2.0	0.068	8.3	LOS A	0.0	0.0	0.00	0.73	49.0
8	т	574	2.0	0.301	25.4	LOS B	10.6	75.4	0.95	0.00	33.
9	R	4	2.0	0.301	35.2	LOS C	10.6	75.4	1.00	1.09	32.5
Approa	ch	674	2.0	0.301	23.1	NA	10.6	75.4	0.81	0.11	35.0
West: V	Veeny St										
10	L	28	2.0	1.717	751.7	LOS F	32.9	234.6	1.00	2.47	2.8
11	т	23	2.0	1.717	750.5	LOS F	32.9	234.6	1.00	2.31	2.8
12	R	52	2.0	1.717	751.8	LOS F	32.9	234.6	1.00	2.27	2.8
Approa	ch	103	2.0	1.717	751.5	LOS F	32.9	234.6	1.00	2.33	2.8
All Vehi	icles	2659	2.0	4.596	449.4	NA	173.4	1234.8	0.61	0.81	4.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Giveway / Yield (Two-Way)

		Demand		Deg	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Tum	Flow veh/h	HV %	Satn v/c	Delay sec	Service	Vehicles veh	Distance m	Queued	Stop Rate per veh	Speed km/h
South: I	Rocky Pt F										
1	L	24	2.0	0.128	8.3	LOS A	0.0	0.0	0.00	1.03	49.0
2 3	т	762	2.0	0.564	30.2	LOS C	12.5	89.2	0.71	0.00	31.4
3	R	61	2.0	0.564	50.9	LOS D	12.5	89.2	1.00	1.21	26.2
Approa	ch	847	2.0	0.564	31.1	NA	12.5	89.2	0.71	0.12	31,3
East: A	ccess										
4	L	108	2.0	4.477	3199.0	LOS F	157.1	1118.6	1.00	4.73	0.7
5	т	23	2.0	4.477	3197.7	LOS F	157.1	1118.6	1.00	4.26	0.7
6	R	170	2.0	4.477	3199.1	LOS F	157.1	1118.6	1.00	4.18	0.7
Approa	ch	301	2.0	4.477	3199.0	LOS F	157.1	1118.6	1.00	4.38	0.7
North: F	Rocky Pt F	₹d									
7	L	121	2.0	0.385	8.3	LOS A	0.0	0.0	0.00	0.99	49.0
8	т	1299	2.0	0.385	4.6	LOS A	7.9	56.1	0.50	0.00	51.0
9	R	18	2.0	0.385	17.1	LOS B	7.9	56.1	0.95	1.12	44.3
Approa	ch	1438	2.0	0.385	5.1	NA	7.9	56.1	0.46	0.10	50.7
West: V	Veeny St										
10	L	24	2.0	1.250	403.5	LOS F	16.2	115.0	1.00	1.86	5.0
11	т	24	2.0	1.250	402.2	LOS F	16.2	115.0	1.00	1.61	4.9
12	R	27	2.0	1.250	403.6	LOS F	16.2	115.0	1.00	1.60	4.9
Approa	ch	75	2.0	1.250	403.1	LOS F	16.2	115.0	1.00	1.68	4.9
All Vehi	cles	2661	2.0	4.477	385.8	NA	157.1	1118.6	0.62	0.63	5.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Signals - Fixed Time Cycle Time = 100 seconds (User-Given Cycle Time)

Mov ID	Turn	Demand Flow	HV	Deg Satn	Average Delav	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	V/C	sec		veh	m	and the second	per veh	km/i
South: F	Rocky Pt F	Rd		10000000000	NY STREET						Witten
1	L	59	2.0	0.763	10.1	LOS A	8.5	60.3	0.20	1.03	47.9
2	т	1370	2.0	0.763	2.0	LOS A	8.5	60.3	0.21	0.19	55
3	R	122	2.0	0.763	10.5	LOS A	4.7	33.4	0.24	0.95	47.4
Approac	ch	1551	2.0	0.763	2.9	LOS A	8.5	60.3	0.21	0.28	54.
East: Ac	cess										
4	L	105	2.0	0.310	43.9	LOS D	5.2	37.1	0.88	0.79	27.4
5	т	21	2.0	0.310	35.6	LOS C	5.2	37.1	0.88	0.71	27.8
6	R	205	2.0	0.725	49.4	LOS D	9.6	68.3	1.00	0.85	25.4
Approac	ch	331	2.0	0.725	46.8	LOS D	9.6	68.3	0.95	0.82	26.1
North: R	Rocky Pt R	d									
7	L	96	2.0	0.181	12.8	LOS A	0.9	6.2	0.17	0.72	44.
8	т	574	2.0	0.761	6.6	LOS A	11.2	80.1	0.43	0.39	49.0
9	R	4	2.0	0.761	14.8	LOS B	11.2	80.1	0.44	1.01	44.
Approac	ch	674	2.0	0.761	7.5	LOS A	11.2	80.1	0.39	0.44	48.
West: W	leeny St										
10	L	28	2.0	0.255	59.9	LOS E	1.4	9.9	0.99	0.72	22.6
11	т	23	2.0	0.688	54.8	LOS D	4.0	28.1	1.00	0.82	22.3
12	R	52	2.0	0.688	63.2	LOS E	4.0	28.1	1.00	0.82	22.1
Approac	ch	103	2.0	0.688	60.4	LOS E	4.0	28.1	1.00	0.79	22.3
All Vehic	cles	2659	2.0	0.763	11.8	LOSA	11.2	80.1	0.38	0.41	44.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P1	Across S approach	53	38.7	LOS D	0.1	0.1	0.88	0.88
P3	Across E approach	53	20.5	LOS C	0.1	0.1	0.64	0.64
P5	Across N approach	53	38.7	LOS D	0.1	0.1	0.88	0.88
P7	Across W approach	53	20.5	LOS C	0.1	0.1	0.64	0.64
All Ped	estrians	212	29.6	LOS C			0.76	0.76

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Signals - Fixed Time Cycle Time = 100 seconds (User-Given Cycle Time)

	and the second	Demand	LINT.	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Tum	Flow veh/h	HV %	Satn v/c	Detay sec	Service	Vehicles veh	Distance	Queued	Stop Rate per veh	Speed km/r
South: F	Rocky Pt I		name in			i iller so all					
1	L	24	2.0	0.180	9.4	LOS A	0.6	4.1	0.08	1.00	48.0
2	т	762	2.0	0.760	1.3	LOS A	4.0	28.4	0.15	0.12	56.7
3	R	61	2.0	0.760	9.6	LOS A	4.0	28.4	0.17	1.01	48.1
Approac	ch	847	2.0	0.760	2.2	LOS A	4.0	28.4	0.15	0.21	55.7
East: Ac	cess										
4	L	108	2.0	0.394	48.1	LOS D	5.8	41,1	0.93	0.80	26.0
5	т	23	2.0	0.394	39.8	LOS C	5.8	41,1	0.93	0.75	26.3
6	R	170	2.0	0.801	58.1	LOS E	8.4	60.1	1.00	0.97	23.0
Approa	ch	301	2.0	0.801	53.1	LOS D	8.4	60.1	0.97	0.89	24.3
North: F	Rocky Pt F	Rd									
7	L	121	2.0	0.797	10.9	LOS A	7.7	54.9	0.22	0.97	47.1
8	т	1299	2.0	0.797	2.7	LOS A	7.7	54.9	0.22	0.21	54.7
9	R	18	2.0	0.797	11.0	LOS A	7.3	51.7	0.22	1.05	47
Approa	ch	1438	2.0	0.797	3.5	LOS A	7.7	54.9	0.22	0.28	53.8
West: V	Veeny St										
10	L	24	2.0	0.218	59.7	LOS E	1.2	8.5	0.98	0.71	22.7
11	т	24	2.0	0.484	53.0	LOS D	2.6	18.5	1.00	0.74	22.9
12	R	27	2.0	0.484	61.3	LOS E	2.6	18.5	1.00	0.74	22.7
Approa	ch	75	2.0	0.484	58.1	LOS E	2.6	18.5	0.99	0.73	22.8
All Vehi	cles	2661	2.0	0.801	10.2	LOSA	8.4	60.1	0.30	0.34	46.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Constant a subset		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
Mov ID	Description	Flow ped/h	Delay sec	Servica	Pedestrian ped	Distance m	Queued	Stop Rate per ped
P1	Across S approach	53	42.3	LOS E	0.1	0.1	0.92	0.92
P3	Across E approach	53	16.8	LOS B	0.1	0.1	0.58	0.58
P5	Across N approach	53	42.3	LOS E	0.1	0.1	0.92	0.92
P7	Across W approach	53	16.8	LOS B	0.1	0.1	0.58	0.58
All Ped	estrians	212	29.6	LOS C			0.75	0.75

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Processed: Monday, 19 August 2013 9:29:18 AM SIDRA INTERSECTION 5.1.13.2093 Project: F:\WORK13\13091 - 152-206 ROCKY POINT ROAD, KOGARAH\MODELLING\ROCKY POINT RD

# APPENDIX D

## EXTRACTS FROM RTA H.D. RESIDENTIAL STUDY

## 4.2 SUB-REGIONAL CENTRES - BRIGHTON-LE-SANDS

## 4.2.4 <u>'Mornington'</u>

### **General Site Description**

The 'Mornington' apartment building is located in Princes Street between Grand Parade and Moate Avenue immediately to the north of the small Brighton-Le-Sands shopping centre. The building is situated approximately 200 metres from the waterfront of Botany Bay and the bus services running along Grand Parade (to the city) and Bay Street (to Rockdale Railway Station). The surrounding landuse is comprised of high and medium density residential developments and the large 'Brighton Resort' hotel and commercial centre. Vehicle access to the residents (secure) and visitors parking areas is provided on Princes Street. On-street parking in Princes Street is limited, particularly at night, due to the activity associated with the 'Brighton Resort' and nearby restaurants however public parking is available within the 'Brighton Resort' complex.

Number of Levels	; 7
Number of Units	: 27
Number of Bedrooms	: 14 x 2 bed, 13 x 3 bed
Parking on-site	: 35
Resident -	27
Visitor -	8
Other Uses -	Nil
Type of Resident	: 20 owner occupied, 7 leased
Public Transport	: Good access to bus services



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			HIGH	RISE RE	SIDENT	IAL						
			'N	<b>IORNIN</b>	GTON'							
	Land and the second		VEHIC	LE TRIP G	ENERAT	ION	1				-	
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	RESIDENT	VISITOR/ SERVICE	PU/SD TAXI	RESIDENT	VISITOR/ SERVICE	PU/SD TAXI		WEEKC	DAY		SATUR	DAY
07.00 - 08.00	1	0	0			-		1			-	1
08.00 - 09.00	5	0	0	2	0	0		5			2	
09.00 - 10.00	7	0	0	2	0	0		7			2	
10.00 - 11.00	-	-	-	7	0	0					7	
11.00 - 12.00	-	-	- 11	2	2	2		-			6	
12.00 - 13.00	-	-	-	2	0	0		-			2	
16.00 - 17.00	0	0	0	-	-	-		0			-	
17.00 - 18.00	4	0	0	-	•	-		4		-	-	
18.00 - 19.00	6	0	0	-	-	-		6			-	
PEAK HOUR (BY	15 MINS)						IN	ουτ	TOT	IN	ουτ	TOT
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PM 18.00 - 19.00	6	0	0	-	-	-	4	2	6	-	-	-
SATURDAY 10.00 - 11.00	-	-	-	7	0	0	-	-	-	2	5	7
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TIME			WEEKD	AY				SATI	URDAY	<i>,</i>		
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12.00 - 13.00			-		-	7		5	;		12	
16.00 - 17.00		8	1		9	•		•			-	
17.00 - 18.00		0	4		4						-	
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°M 8.00 - 19.00		3	10		13	-		-		-		
ATURDAY 0.30 - 11.30		-	-		-	9		6			15	

					RESIDENTI NGTON'			
					K ACCUMU			
	<u> </u>		<u>RESIDENT</u>	Weekd			Saturd	<u></u>
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07.00		07.15	0		(24*) 23	-	-	-
07.15		07.30	0	0	23	-		-
07.30		07.45	0	0	23	-	<u> </u>	-
07.45		08.00	0	0	23	· .		-
08.00	<u></u>	08.15	0	1 1	22	0	1	(18*) 17
08.15	-	08.30	0	2	20	0	0	17
08.30		08.45	0		19	0	1	16
08.45	-	09.00	0	1 1	18	0	0	16
09.00	-	09.15	1	2	17	0	0	16
09.15	-	09.30	0	1	16	0	0	16
09.30		09.45		2	15	0	0	16
09.45		10.00	0	0	15	0	0	16
10.00	-	10.15	·····	-	-	0	1	15
10.15	-	10.30	-			0	1	14
10.30		10.45		-		0	0	14
10.45	-	11.00				0	2	12
11.00		11.15		-	-	0	1	11
11.15		11.30				0	0	11
11.30		11.45		-	-	0	0	11
11.45		12.00			-	0	1	10
12.00	_	12.15		-	•	0	0	10
12.15	-	12.30		_		0	0	10
12,30		12.45			<u> </u>	0	0	10
12.45	 -	13.00			-	0	0	10
16.00	-	16.15	0	0	(16*) 16	-		
16.15	_	16.30	0	0	16			-
16.30	-	16.45	0	0	16	-	-	
16.45	-	17.00	0	0	16	-	-	<b>-</b>
17.00	 -	17.15	1	0	17		- 1	
17.15	-	17.30	1	0	18	-	-	
7.30	-	17.45	1	0	19	-	-	-
7.45	-	18.00	0	1	18	•	-	-
8.00	-	18.15	0	1	17	-	•	
8.15	-	18.30	2	1	18	-	<u> </u>	-
8.30	-	18.45	1	0	19		-	
8.45	-	19.00	1	0	20	-		-
	u ico D		ing by Census			um Visitor P	arking by s	urvev
	GUHER	asidem sork 32			maxill.	9 - Sunda		

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\* Number of vehicles at commencement of survey

## 4.2 SUB-REGIONAL CENTRES - BRIGHTON-LE-SANDS

## 4.2.5 <u>`Bankswater'</u>

### General Site Description

The 'Bankswater' apartment building is located in Princes Street between Grand Parade and Moate Avenue immediately to the north of the small Brighton-Le-Sands shopping centre. The building is situated approximately 200 metres from the waterfront of Botany Bay and the bus services running along Grand Parade (to the city) and Bay Street (to Rockdale Railway Station). The surrounding landuse is comprised of high and medium density residential developments and the large 'Brighton Resort' hotel and commercial centre. Vehicle access to the residents (secure) and visitors parking areas is provided on Princes Street. On-street parking in Princes Street is limited, particularly at night, due to the activity associated with the 'Brighton Resort' and nearby restaurants however public parking is available within the 'Brighton Resort' complex.

Number of Levels	: 8
Number of Units	: 40
Number of Bedrooms	: 24 x 2 bed, 16 x 3 bed
Parking on-site	: 52
Resident - Visitor -	42 10
Other Uses -	Nii
Type of Resident	: N/A
Public Transport	: Good access to bus services



	• • • • • • • • • • • • • • • • • • • •		HIGH	RISE RE	SIDENT	IAL			·•·			
			"E	BANKSW	ATER'					<u></u>		
			VEHIC	LE TRIP G	ENERATI	ON						
TIME	<u> </u>	NEEKDAY		S	ATURDAY				TO1			<u></u>
	RESIDENT	VISITOR/ SERVICE	PU/SD TAXI	RESIDENT	VISITOR/ SERVICE	PU/SD TAXI		WEEKDA	.Y		SATURE	)AY 
07.00 - 08.00	9	0	0	-	-			9				
08.00 - 09.00	6	1	6	6	0	4		13			10	,
09.00 - 10.00	4	0	0	6	1	2	-		<del>.</del>		9	
10.00 - 11.00	-	-	•	3	0	0	•				3	
11.00 - 12.00	-	-	-	3	2	2		-	- <u></u>		7	<u> </u>
12.00 - 13.00	-	-	-	1	5	2	-		- *····		8	
16.00 - 17.00	3	1	0	-	-	-		4				L
17.00 - 18.00				2 -		-		13				
18.00 - 19.00	6	3	0	-	-	-		9				
PEAK HOUR (BY	15 MINS)		<del></del>				IN	001	TOT	IN	ол	TOT
AM 07.30 - 08.30	9	0	6	-		-	5	10	15	-	-	
PM 18.00 - 19.00	6	4	6	-	-	-	12	4	16	-	-	-
SATURDAY 08.30 - 09.30	-	-	-	8	0	0	-	-	-	7	1	8
			PERSO	n trip ge	NERATIO	DN						
TIME			WEEKD	YAY				SATI	JRDAY			
	-	IN	OUT	T <sup>i</sup>	OTAL	IN		. 0	UT		TOTA	L
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08.00 - 09.00		3	7		10	8		1	0		18	
09.00 - 10.00		3	3		6	5		1	3		18	
10.00 - 11.00		-			-	· 8		1.	4		22	
11.00 - 12.00		- [			-			6			14	
12.00 - 13.00		-			-	10		7			17	
16.00 - 17.00		11	3		14						-	
17.00 - 18.00		10	5		15				-	<b>n</b>		
18.00 - 19.00		9	14		23	-		-				
PEAK HOUR (BY 1	5 MINS)						<u> </u>					
AM 07.45 - 08.45		5	12		17	-					-	
PM 18.00 - 19.00		9	14		23	-		-				
SATURDAY 10.15 - 11.15		-	-		-	13		16	5		29	

			HIGH	I RISE F	RESIDENTI	AL				
	·		6	BANKS	WATER'			. <u> </u>		
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07.00	-	07.15	0	1	(28*) 27		-	<u> </u>		
07.15	•	07.30	0	0	27	<u> </u>	<u> </u>	<u> </u>		
07.30	÷	07.45	2	2	27			<u> </u>		
07.45		08.00	0	3	24	<u> </u>				
08.00	-	08.15	0	0	24	0	0	(27*) 27		
08.15	-	08.30	0	2	22	0	0	27		
08.30	-	08.45	0	3	19	0	3	24		
08.45	•	09.00	0	0	19	0	0	24		
09.00	-	09.15	0	0	19	0	3	21		
09.15	-	09.30	0	1	18	0	1	20		
09.30	<b>-</b>	09,45	0	]	17	0	0	20		
09.45	-	10.00	0	0	17	1	0	21		
10.00	-	10.15	-	-	<u>.</u>	0	0	21		
10,15	-	10.30	-	-		0	0	21		
10.30		10.45	-	-	-	0	0	21		
10.45	-	11.00	-		-	0	1	20		
11.00	-	11.15	•	-	-	0	1	19		
11.15	-	11.30	-	-	-	0	1	18		
11.30	-	11.45	-	-	-	0	0	18		
11.45	-	12.00	-	-	-	0	0	18		
12.00	-	12.15	-	-	-	0	0	18		
12,15	-	12,30	-	-	-	0	0	18		
12.30	-	12.45	-	-	-	0	0	18		
12.45	-	13.00	-	-	-	0	0	18		
16.00	•	16.15	1	0	(16*) 17	-	-	-		
16.15	-	16.30	1	1	17	-	-			
16.30	-	16.45	0	0	17	-	-			
16.45	-	17.00	0	0	17		-	-		
17.00	-	17.15	1	0	18	-	-			
17.15	-	17.30	1		18	<u> </u>	-			
17.30	-	17.45	0	1	17		-	•		
17.45	-	18.00	3	0	20			-		
18.00	•	18.15	3	0	23					
18.15	<u>-</u>	18.30	2	0	25	-	-	•		
18.30	-	18.45	0	1	24	-	-			
18.45	•	19.00	0	0	24	-	-	-		
Maximu	im Re	sident Parki	ng by Censu	s Data	Maximu	m Visitor P	arking by	Survey		
		46			6 - Sunday 15.00					

\* Number of vehicles at commencement of survey

# APPENDIX E

## **RMS WARRANTS**



## 2.1 INTRODUCTION

This section describes the general warrants for the installation of traffic signals. It must be emphasised that these are only a guide. If a site satisfies the warrants, it does not necessarily mean that traffic signals are the best solution. All traffic data should be analysed and alternative treatments considered to determine the optimum solution (see Section 4 of the *Road Design Guide*). Traffic signals are sometimes installed due to public pressure or an administrative directive irrespective of the general warrants.

## 2.2 FACTORS INFLUENCING THE PROVISION OF TRAFFIC SIGNALS

Traffic signals are usually installed at an intersection:

- to provide traffic control at a site with a traffic capacity or road safety problem
- to control conflicting movements with high traffic flows
- to facilitate access to and from local areas in a major/minor road system, including pedestrian movements
- as part of an area wide system of traffic management

A side effect of signalisation is that the traffic flow on a major road is broken up into platoons. This assists nearby pedestrians to cross the major road and vehicles in nearby side streets to cross or enter the major road.

Factors influencing the provision of traffic signals include:

- traffic flows
- traffic conflicts
- traffic accident statistics
- pedestrian requirements
- access to major roads
- cost of installation
- availability of funds
- maintenance costs
- practicality
- feasibility
- the signposted speed limit is not more than 80km/h

General warrants are given in the following sub-sections. The figures stated should only be used as a guide and each intersection should be considered in more detail before being accepted for signal design.

## 2.3 SIGNALISED INTERSECTIONS

As a guide, a signalised intersection may be considered if one of the following warrants is met.

#### (a) Traffic demand:

For each of four one-hour periods of an average day:

- (i) the major road flow exceeds 600 vehicles/hour in each direction; and
- (ii) the minor road flow exceeds 200 vehicles/hour in one direction.

OR



### (b) Continuous traffic:

- For each of four one-hour periods of an average day:
  - ) the major road flow exceeds 900 vehicles/hour in each direction; and
- (ii) the minor road flow exceeds 100 vehicles/hour in one direction; and
- (iii) the speed of traffic on the major road or limited sight distance from the mino road causes undue delay or hazard to the minor road vehicles; and
- (iv) there is no other nearby traffic signal site easily accessible to the minor road vehicles.

### OR

- (c) Pedestrian safety:
  - For each of four one-hour periods of an average day:
  - (i) the pedestrian flow crossing the major road exceeds 150 persons/hour; and
  - (ii) the major road flow exceeds 600 vehicles/hour in each direction or, where there is a central median of at least 1.2 m wide, 1000 vehicles/hour in each direction.

### OR

- (d) Pedestrian safety high speed road:
  - For each of four one-hour periods of an average day:
    - (i) the pedestrian flow crossing the major road exceeds 150 persons/hour; and
    - the major road flow exceeds 450 vehicles/hour in each direction or, where there is a central median of at least 1.2 m wide, 750 vehicles/hour in each direction; and
    - (iii) the 85th percentile speed on the major road exceeds 75 km/h.

#### OR

- (e) Crashes:
  - (i) The intersection has been the site of an average of three or more reported tow-away or casualty traffic accidents per year over a three year period, where the traffic accidents could have been prevented by traffic signals; and
  - (ii) the traffic flows are at least 80% of the appropriate flow warrants.

## 2.4 SIGNALISED MARKED FOOT CROSSINGS AT INTERSECTIONS

A signalised marked foot crossing must be provided on each leg of a signalised intersection (including T Junctions), in a built-up area, except in the following circumstances:

(a) There are significant road safety implications:

(i) there is insufficient sight distance (see Section 4 of the Road Design Guide); or

- (ii) there is adverse road geometry (see Section 4 of the Road Design Guide).
- (b) There are significant adverse transport efficiency implications
  - (i) there is an unacceptable increase in delay and degree of saturation which must be substantiated by intersection modelling; or